

① 仮定断面 H-400×300×12×18 (SN400)

スパン $l = 9600$, $l_b = 4800$ (mm)

<断面性能>

$$I = \frac{18 \times 300^3}{12} \times 2 + \frac{364 \times 12^3}{12}$$

$$= 8.105 \times 10^7 \text{ (mm}^4\text{)}$$

$$Z = \frac{I}{\frac{400}{2}} = 5.403 \times 10^5 \text{ (mm}^3\text{)}$$

$$A = 18 \times 300 \times 2 + 12 \times 364$$

$$= 15168 \text{ (mm}^2\text{)}$$

$$i_{yz} = \sqrt{\frac{I}{A}} = 73.099 \text{ (mm)}$$

曲げ応力に関する断面二次半径 i

$$i = \sqrt{\frac{\frac{18 \times 300^3}{12}}{18 \times 300 + \frac{1}{6} \times 12 \times 364}} = 81.296 \text{ (mm)}$$

<中厚比の検定>

$$\text{フランジ} : \frac{b}{t_f} = \frac{150}{18} = 8.33 < 9 \text{ (ok)}$$

$$\text{ウェブ} : \frac{d}{t_w} = \frac{364}{12} = 30.3 < 60 \text{ (ok)}$$

<横補剛の検定>

$$\lambda_y = \frac{l}{i_y} = \frac{9600}{73.099} = 131.33 < 170 + 20n \text{ (ok)}$$

<許容曲げ応力度>

$$\text{限界細長比 } \Lambda = \sqrt{\frac{\pi^2 \cdot 205000}{0.6 \times 235}} = 119.789$$

$$\text{細長比 } \frac{l_b}{i} = \frac{4800}{81.296} = 59.044$$

$$f_{b1} = \left\{ 1 - 0.4 \frac{\Lambda^2}{\Lambda^2} \right\} f_t = 141.442$$

$$f_{b2} = \frac{89000}{\frac{4800 \times 4800}{18 \times 300}} = 250.3125 > f_t$$

$$f_b = 156.667 \text{ (N/mm}^2\text{)} \text{ (長期)}$$

①端

$$\sigma_{b1} = \frac{136 \times 10^6}{1640192} = 82.92 \text{ (N/mm}^2\text{)} < f_b \text{ (ok)}$$

②端

$$\sigma_{b2} = \frac{157 \times 10^6}{1640192} = 95.72 \text{ (N/mm}^2\text{)} < f_b \text{ (ok)}$$

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$$\sigma_{b3} = \frac{107 \times 10^6}{1640192} = 65.236 \text{ (N/mm}^2\text{)} < f_b \text{ (ok)}$$

<許容せん断応力度>

$$\text{①端 } \tau_1 = \frac{Q}{A_w} = \frac{50.6 \times 10^3}{12 \times 364} = 11.584$$

$$f_s = \frac{F}{\sqrt{3}} = 135.6 \text{ N/mm}^2 \quad \frac{\tau_1}{f_s} < 1 \text{ (ok)}$$

$$\text{②端 } \tau_2 = 12.592 \quad \frac{\tau_2}{f_s} < 1 \text{ (ok)}$$

A_w : W17" 断面積

