

(1)

$$I_b = \frac{30 \times 40^3}{12} = 160000 \text{ cm}^4$$

$$I_c = \frac{30^4}{12} = 67500 \text{ cm}^4$$

$$\text{剛度 } K_b = \frac{I_b}{l_b} = 400 \text{ cm}^3$$

$$K_c = \frac{I_c}{l_c} = 225 \text{ cm}^3$$

標準剛度  $K_0$  を 25 とすれば

$$\text{剛比 } k_b = \frac{K_b}{K_0} = 16$$

$$k_c = \frac{K_c}{K_0} = 9$$

節点方程式より

$$M_{BA} + M_{BC} = 0$$

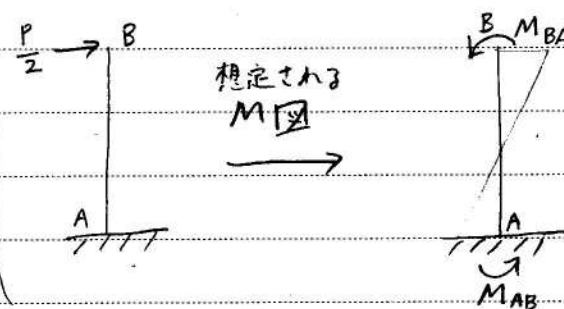
$$66\phi_B + 9\psi = 0 \quad \text{--- ①}$$

層方程式より

$$M_{AB} + M_{BA} = -\frac{P}{2} \cdot h$$

↑ 反時計まわりだから

∵ 逆対称だから それぞれの柱に  $\frac{P}{2}$  ずつ  
力がかかる。



ゆえに

$$9(\phi_B + \psi) + 9(2\phi_B + \psi) = -\frac{40}{2} \cdot 300$$

$$27\phi_B + 18\psi = -6000 \quad \text{--- ②}$$

①, ②より

$$\phi_B = 57.143$$

$$\psi = -419.05$$

$$\left\{ \begin{array}{l} M_{AB} = 9(2\phi_A + \phi_B + \psi) \\ M_{BA} = 9(2\phi_B + \phi_A + \psi) \\ M_{BC} = 16(2\phi_B + \phi_C + \psi) \\ M_{CB} = 16(2\phi_C + \phi_B + \psi) \\ M_{CD} = 9(2\phi_C + \phi_D + \psi) \\ M_{DC} = 9(2\phi_D + \phi_C + \psi) \end{array} \right. \begin{array}{l} \text{左柱} \\ \text{梁} \\ \text{右柱} \end{array}$$

逆対称であるから  $\phi_B = \phi_C$

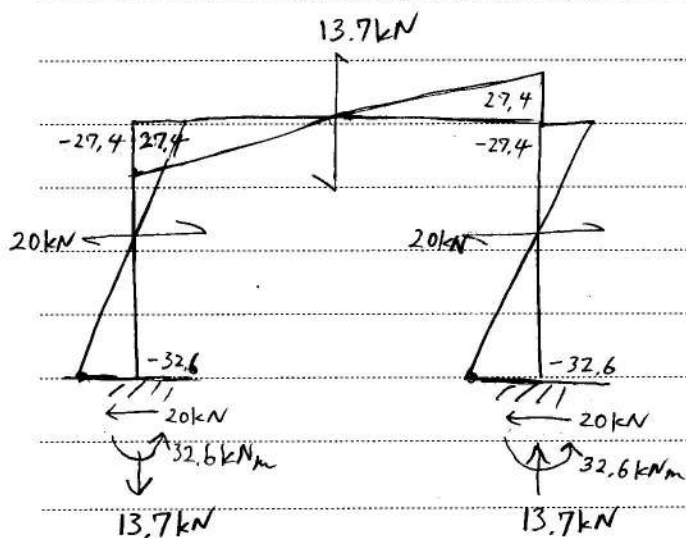
$$\left\{ \begin{array}{l} M_{AB} = 9(\phi_B + \psi) \\ M_{BA} = 9(2\phi_B + \psi) \\ M_{BC} = 48\phi_B \\ M_{CB} = 48\phi_B \\ M_{CD} = 9(2\phi_B + \psi) \\ M_{DC} = 9(\phi_B + \psi) \end{array} \right.$$

授業科目 \_\_\_\_\_ 学科 \_\_\_\_\_ 年次 \_\_\_\_\_ 学籍番号 \_\_\_\_\_ 氏名 No.3 ②

$$\begin{cases} M_{AB} = -3257 \text{ kNcm} = -32.6 \text{ kNm} \\ M_{BA} = -2743 \text{ kNcm} = -27.4 \text{ kNm} \\ M_{BC} = 2743 \text{ kNcm} = 27.4 \text{ kNm} \\ M_{CB} = M_{BC} = 27.4 \text{ kNm} \\ M_{CD} = M_{BA} = -27.4 \text{ kNm} \\ M_{DC} = M_{AB} = -32.6 \text{ kNm} \end{cases}$$

$$\begin{aligned} 2) \psi &= 2EK_0(-3R) \\ -419.05 &= 2 \cdot 1000 \cdot 25 \cdot (-3R) \\ R &= 0.0027937 \text{ (rad)} \\ \frac{\delta}{h} &= R \text{ よ} \\ \delta &= 0.838 \text{ cm} \\ R &< \frac{1}{200} \text{ であるから } \underline{\text{ok}} \end{aligned}$$

M図



$$3) Z = \frac{BD^2}{6} \text{ よ}$$

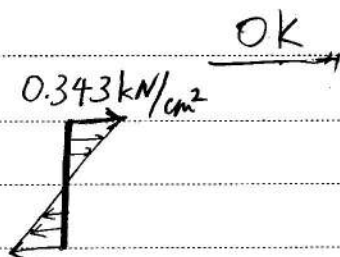
$$Z_b = 8000 \text{ cm}^3$$

$$Z_c = 4500 \text{ cm}^3$$

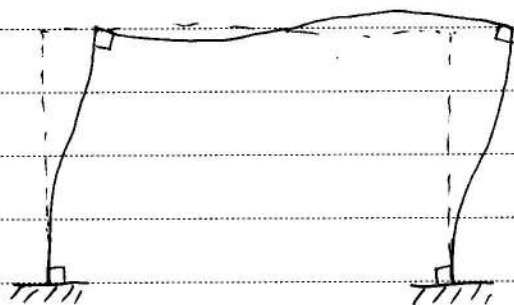
(i) 梁の検討

$$\sigma_b = \frac{M}{Z_b} = \frac{2743}{8000}$$

$$= 0.343 \text{ kN/cm}^2 < 0.6 \text{ kN/cm}^2$$



変形



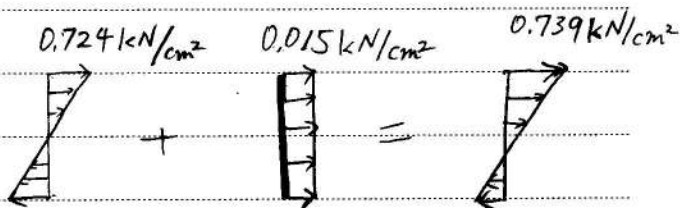
(ii) 柱の検討

$$\sigma_c = \frac{M}{Z_c} + \frac{P}{A}$$

$$= \frac{3257}{4500} + \frac{13.7}{900}$$

$$= 0.739 > 0.6$$

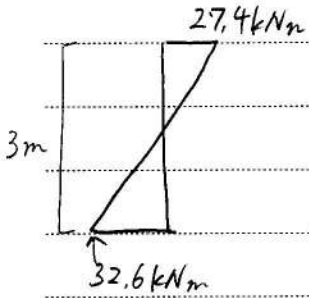
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2) の別解として

弾性曲線式でも求められます。



$$\begin{cases} M(y) = 32.6 - 20y \\ EI\theta(y) = -10y^2 + 32.6y + C_1 \\ EI\delta(y) = -\frac{10}{3}y^3 + 16.3y^2 + C_1y + C_2 \end{cases}$$

$y=0$  のとき  $\theta=0, \delta=0$  より  $C_1 = C_2 = 0$

$y=3$  のとき  $\delta = \frac{-\frac{10}{3} \times 300^3 + 16.3 \times 300^2}{EI}$

$\delta = 0.838 \text{ cm}$